EXPLOTECH

Specialists in Explosives, Blasting and Vibration
Consulting Engineers

Blast Impact Analysis
Uppers Quarry
Part of Lots 119, 120, 136 and 137, Part of the road allowance between Lots 120 and 136 (Geographic Township of Stamford), City of Niagara Falls, Regional Municipality of Niagara

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EXECUTIVE SUMMARY

Explotech Engineering Ltd. was retained in November 2016 to provide a Blast Impact Analysis for the proposed Uppers Quarry located on Part of Lots 119, 120, 136 and 137. The Assessment also takes into account potential extraction of the portion of Upper's Lane and part of the unopened road allowance between Lots 120 and 136 (geographic township of Stamford), where they exist between Thorold Townline Road and Beechwood Road, all in the City of Niagara Falls, Regional Municipality of Niagara.

Vibration levels assessed in this report are based on the Ministry of the Environment, Conservation and Parks (MECP) Model Municipal Noise Control By-law with regard to guidelines for blasting in Mines and Quarries. We have assessed the area surrounding the proposed licence area as it relates to potential damage from blasting operations and compliance with the aforementioned By-law document.

We have inspected the property and reviewed the available site plans. Explotech is of the opinion that the planned aggregate extraction on the proposed property can be carried out safely and within MECP guidelines as set out in NPC 119 of the By-Law.

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INTRODUCTION

Walker Aggregates (Walker) intends to apply for a Class A, Category 2 Licence for the property legally described as Part of Lots 119, 120, 136 and 137, former Township of Stamford, now in the City of Niagara Falls, in the Regional Municipality of Niagara. Two municipal road allowances (Upper's Lane and an unopened road allowance between Part of Lots 120 and 136 in the former Township of Stamford) separate the proposed quarry site into three extraction areas:

i) North Extraction Area: extraction area North of Upper's Lane;
ii) Mid Extraction Area: extraction area South of Upper's Lane and North of the unopened road allowance between Township Lots 120 & 136 in the former Township of Stamford, now in the City of Niagara Falls ("unopened road allowance"); and
iii) South Extraction Area: extraction area South of the unopened road allowance. Part of the road allowance between Lots 120 and 136 (geographic township of Stamford), City of Niagara Falls, Regional Municipality of Niagara.

This Blast Impact Analysis assesses the ability of the proposed licence (whether the two road allowances are ultimately included in the extraction area or not) to operate within the prescribed blast guideline limits as required by the Ontario Ministry of the Environment, Conservation and Parks (MECP).

While not specifically required as part of the scope of the Blast Impact Analysis under the Aggregate Resources Act, this report also touches on the topics of the flyrock and residential water wells for general informational purposes only. Details related to residential water wells are addressed in the hydrogeological report while specific flyrock control is addressed at the operational level given significant influences related to blast design, geology and field accuracy. Additionally, potential impacts on the adjacent TC Energy pipeline, electrical transmission towers, and nearby waterbodies are discussed to confirm compliance with applicable external corporate policies and guidelines.

The proposed Uppers Quarry operation is bounded by farm land to the North and South, Beechwood Road to the East, a residential subdivision (Fernwood) to the Southeast and Thorold Townline Road to the West. A residential and employment subdivision (Rolling Meadows) has been approved West of Thorold Townline Road. The extraction areas will be accessed via Thorold Townline Road and Upper's Lane.

The proponent currently owns the lands housing the residences designated as 5872 Thorold Townline Road and 5497 Beechwood Road. The closest sensitive receptor not owned by the proponent is the property designated as 5329 Beechwood Road.

This Blast Impact Analysis has been prepared based on the MECP Model Municipal Noise Control By-law with regard to Guidelines for Blasting in Mines and Quarries (NPC 119).

Given that mining operations have not been undertaken in the past on this property, site-specific blast monitoring data is not available. We have therefore applied data generated across a spectrum of quarries and construction projects which provides a conservative approximation of anticipated vibration levels from the operation. It has been our experience that this data represents a conservative starting point for blasting operations. It is a recommendation of this report that a vibration monitoring program be initiated on-site upon the commencement of blasting operations and maintained for the duration of all blasting activities to confirm compliance with MECP guideline limits for ground vibration and overpressure based on actual measurements taken during blast times.

EXISTING CONDITIONS

The licenced area for the proposed Uppers Quarry encompasses a total area of approximately 103.6HA. The total extraction area is approximately 89.1HA.

The site is separated into eight (8) extraction phases. The phases are designated as 1a, 1b, 2a, 2b, 3a, 3b, 4 and 5 with phased extraction progressing initially from the Northwest corner of Phase 1a along the West end of the property (Refer to Appendix A).

The topography of the proposed licence area is generally lowest in the North portion of the site at an elevation in the order of 177masl rising towards the West and South with the highest elevations (185masl) lying in the middle of the West boundary of the site (Phase 1b). The design final quarry floor elevation is 141masl –148.7masl leading to the likely execution of 2 to 3 benches to achieve final grade.

The lands surrounding the proposed licence area are largely characterized by agricultural areas with a limited number of residential structures within 500m aside from a subdivision located Southeast of the boundary limits. All sensitive receptors within 500m of the licence boundary are as follows:

Address	Distance to Receptor from Licence Boundary (m)	Direction from Licence Boundary
9337 Beaversdam Road	475	North
9417 Beaversdam Road	447	North
9582 Beaversdam Road	151	North
9722 Beaversdam Road	234	North
10138 Beaversdam Road	442	North
10148 Beaversdam Road	184	North
5329 Beechwood Road	63	East
5584 Beechwood Road	81	Southeast
5769 Beechwood Road	287	Southeast
5821 Beechwood Road	360	Southeast
9312 Madison Crescent	417	Southeast
9315 Madison Crescent	445	Southeast
9324 Madison Crescent	404	Southeast
9325 Madison Crescent	434	Southeast

Address	Distance to Receptor from Licence Boundary (m)	Direction from Licence Boundary
9336 Madison Crescent	390	Southeast
9337 Madison Crescent	423	Southeast
9349 Madison Crescent	415	Southeast
9352 Madison Crescent	370	Southeast
9361 Madison Crescent	407	Southeast
9366 Madison Crescent	354	Southeast
9373 Madison Crescent	391	Southeast
9380 Madison Crescent	338	Southeast
9385 Madison Crescent	371	Southeast
9397 Madison Crescent	351	Southeast
9409 Madison Crescent	334	Southeast
9421 Madison Crescent	316	Southeast
9433 Madison Crescent	299	Southeast
9445 Madison Crescent	280	Southeast
9457 Madison Crescent	260	Southeast
5599 Osprey Avenue	251	Southeast
5607 Osprey Avenue	259	Southeast
5610 Osprey Avenue	311	Southeast
5615 Osprey Avenue	271	Southeast
5622 Osprey Avenue	323	Southeast
5623 Osprey Avenue	284	Southeast
5631 Osprey Avenue	290	Southeast
5632 Osprey Avenue	331	Southeast
5639 Osprey Avenue	299	Southeast
5642 Osprey Avenue	341	Southeast
5647 Osprey Avenue	311	Southeast
5652 Osprey Avenue	350	Southeast
5655 Osprey Avenue	321	Southeast
5663 Osprey Avenue	333	Southeast
5668 Osprey Avenue	362	Southeast
5671 Osprey Avenue	339	Southeast
5679 Osprey Avenue	350	Southeast
5687 Osprey Avenue	362	Southeast
5695 Osprey Avenue	374	Southeast

Address	Distance to Receptor from Licence Boundary (m)	Direction from Licence Boundary
5703 Osprey Avenue	383	Southeast
5711 Osprey Avenue	393	Southeast
5719 Osprey Avenue	404	Southeast
5727 Osprey Avenue	415	Southeast
5735 Osprey Avenue	424	Southeast
5743 Osprey Avenue	438	Southeast
5751 Osprey Avenue	448	Southeast
5759 Osprey Avenue	459	Southeast
5767 Osprey Avenue	470	Southeast
5772 Osprey Avenue	499	Southeast
5775 Osprey Avenue	480	Southeast
5783 Osprey Avenue	490	Southeast
9245 Shoveller Drive*	410	Southeast
9314 Shoveller Drive	494	Southeast
9324 Shoveller Drive	488	Southeast
9334 Shoveller Drive	478	Southeast
9344 Shoveller Drive	467	Southeast
9354 Shoveller Drive	460	Southeast
9364 Shoveller Drive	450	Southeast
9374 Shoveller Drive	443	Southeast
9385 Shoveller Drive	392	Southeast
9394 Shoveller Drive	428	Southeast
9395 Shoveller Drive	383	Southeast
9404 Shoveller Drive	423	Southeast
9405 Shoveller Drive	374	Southeast
9414 Shoveller Drive	416	Southeast
9424 Shoveller Drive	412	Southeast
9434 Shoveller Drive	405	Southeast
9446 Shoveller Drive	400	Southeast
9045 Eagle Ridge Drive	457	Southeast
9355 Eagle Ridge Drive	494	Southeast
9365 Eagle Ridge Drive	481	Southeast
9375 Eagle Ridge Drive	469	Southeast
9385 Eagle Ridge Drive	471	Southeast

Address	Distance to Receptor from Licence Boundary (m)	Direction from Licence Boundary
9395 Eagle Ridge Drive	464	Southeast
9415 Eagle Ridge Drive	448	Southeast
9425 Eagle Ridge Drive	445	Southeast
9435 Eagle Ridge Drive	443	Southeast
9440 Eagle Ridge Drive*	484	Southeast
9445 Eagle Ridge Drive	436	Southeast
9461 Eagle Ridge Drive	427	Southeast
9484 Eagle Ridge Drive	480	Southeast
9490 Eagle Ridge Drive	478	Southeast
9494 Eagle Ridge Drive	477	Southeast
9500 Eagle Ridge Drive	474	Southeast

*Multiple units located at address, closest unit to licence boundary included above

Table 1: Sensitive Receptors within 500m of the Licence Boundary

The properties on the proposed licenced land (5205 and 5497 Beechwood Road, 5872 Thorold Townline Road, 9764 Upper's Lane, 9903 Upper's Lane and 10200 Upper's Lane) are within the proposed extraction limits. Upon commencement of extraction, the use of these structures will be such that they do not qualify as sensitive receptors.

The utility buildings (4832 Thorold Townline Road) located directly adjacent the proposed licence do not qualify as sensitive receptors as defined by the MECP (refer to Appendix E for Definitions) but will remain in place and operational for the foreseeable future. In order to safeguard the integrity of this structure, we recommend that vibrations at the utility buildings be maintained below 50mm/s in accordance with research performed by the United States Bureau of Mines (USBM RI8507). It is a recommendation of this report that this structure shall be monitored for ground vibration when vibration calculations suggest vibrations in excess of 35mm/s.

PROPOSED MINERAL EXTRACTION

An existing watercourse runs North-South through Phase 3a, Phase 4 and Phase 5 of the extraction limits. The proposed existing watercourse realignment will relocate the watercourse inside the excavation limits on the Western limits of extraction inside Phase 1b and 2b.

The extraction operations will begin with a sinking cut at the Northwest portion of Phase 1a (Mid Extraction Area) and will retreat Easterly and Southerly through Phase 1a. Phase 1b is located on the Western edge of Phase 1a and borders the Western most boundary for the proposed licence. In the Mid Extraction Area, Phase 1a will be extracted to a final floor elevation of 141 – 145masl and given existing topography of 181 – 185masl, it is anticipated that Phase 1a extraction will take place in 2 – 3 benches. Phase 1b will be extracted to a depth of no greater than 155masl in order to facilitate the construction of the new realigned watercourse. The Phase 1a and Phase 1b areas are also located in the South Extraction area separated by a road allowance not owned by the proponent. As a result, an additional sinking cut in the Northwest corner of Phase 1a (South Extraction Area) will be required. Phase 1a (South Extraction Area) will retreat Easterly and Southerly through Phase 1a (South Extraction Area). Phase 1b (South Extraction Area) is located on the Western edge of Phase 1a and borders the Western most boundary for the proposed licence. In the South Extraction Area, Phase 1a will be extracted to a final floor elevation of 141 – 145masl and given existing topography of 182 – 185masl, it is anticipated that Phase 1a (South Extraction Area) extraction will take place in 2 – 3 benches. Phase 1b (South Extraction Area) will be extracted to a depth of no greater than 155masl in order to facilitate the construction of the new realigned watercourse.

Phase 2a (North Extraction Area) is located North of Phase 1a and will begin with a sinking cut at the Southwest corner of Phase 2a and will retreat Easterly and Northerly through Phase 2a. Phase 2b is located on the Western and Northern edge of Phase 2a and borders the Western most boundary for the proposed licence. Phase 2a will be extracted to a final floor elevation of 144 – 145masl and given existing topography of 180 – 182masl, it is anticipated that Phase 2a extraction will take place in 2 – 3 benches. Phase 2b will be extracted to a depth no greater than an elevation of 155masl in order to facilitate the construction of the new realigned watercourse whereas the watercourse alignment transition area found at the North end of Phase 2b will only be extracted to a depth of no greater than 174masl in order to facilitate the construction of the new realigned watercourse.

Phase 3a (North Extraction Area) is located to the East of Phase 2a and is currently traversed by the existing watercourse. The existing watercourse is to be relocated to the West perimeter of the site after the successful completion of the Phase 1b and 2b extraction.

A sinking cut in the Southwest corner of Phase 3b will initiate extraction with a Northerly and Easterly retreat. Extraction for Phase 3a will leverage the face created by extraction in Phase 2a with a Easterly retreat. ***

Phase 3a/3b will be extracted to a final floor elevation of 146masl – 149masl and given existing topography of 177masl – 183masl, it is anticipated that Phase 3 extraction will take place in 2 – 3 benches.

Phase 4 is located in the middle of the proposed licence area (Mid Extraction Area) and progresses from the Eastern Phase 1a limit (Mid Extraction Area) Easterly to the East proposed extraction limits. Extraction of Phase 4 will utilize the Eastern face of Phase 1a and will be extracted in 2 – 3 benches to a depth ranging from 141masl – 149masl given existing topography in Phase 4 of 177masl - 184masl.

Phase 5 is located in the South of the proposed licence area (South Extraction Area) and progresses from the Eastern Phase 1a limit (South Extraction Area) Easterly to the South and East extraction limits. Extraction of Phase 5 will utilize the Eastern face of Phase 1a and will be extracted in 2 – 3 benches to a depth ranging from 141masl – 149masl given existing topography in Phase 5 of 178masl - 182masl.

All extraction phases bordering the perimeter limits of the quarry (namely phases 1b, 2b, and 3a) will have a catchment bench located at elevation 149masl – 155masl depending on the phase.

As quarry operations migrate across the property, the closest sensitive receptors to the required blasting operations will vary. The closest receptor to the proposed Upper's Quarry licence area over the life of the quarry is 5329 Beechwood Road at a distance of 63m from the licence boundary.

***Alternatively, if the existing watercourse is realigned prior to the extraction operations in Phase 3, extraction may begin in Phase 3a. Phase 3a will leverage the existing Phase 2a face and retreat Easterly. Phase 3b will then leverage the Phase 3a face and retreat Easterly. If this alternate extraction proceeds, the conclusions of this report are unchanged.

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BLAST VIBRATION AND OVERPRESSURE LIMITS

The Ontario MECP guidelines for blasting in quarries are among the most stringent in North America.

Studies by the U.S. Bureau of Mines have shown that normal temperature and humidity changes can cause more damage to residences than blast vibrations and overpressure in the range permitted by the MECP. The guideline limits set by the MECP are as follows.

Vibration____________12.5mm/sec Peak Vector Sum (PVS)

Overpressure________128 dBL Peak Sound Pressure Level (PSPL)

The above guidelines apply when blasts are being monitored. Cautionary levels are slightly lower and apply when blasts are not monitored on a routine basis. It is a recommendation of this report that all blasts at the operation be monitored to quantify and record ground vibration and overpressure levels employing a minimum of two (2) digital seismographs, one installed at the closest sensitive receptor in front of the blast, or closer, and a second installed at the closest sensitive receptor behind the blast, or closer.

BLAST MECHANICS AND DERIVATIVES

The detonation of explosives within a borehole results in the development of very high gas and shock pressures. This energy is transmitted to the surrounding rock mass, crushing the rock immediately surrounding the borehole (approximately 1 borehole radius) and permanently distorts the rock to several borehole diameters (5-25, depending on the rock type, prevalence of joint sets, etc.).

The intensity of this stress wave decays quickly so that there is no further permanent deformation of the rock mass. The remaining energy from the detonation travels through the unbroken material in the form of a pressure wave or shock front which, although it causes no plastic deformation of the rock mass, is transmitted in the form of vibrations.

Particle velocity is the descriptor of choice when dealing with vibrations because of its superior correlation with the appearance of cosmetic cracking. As such, for the purposes this report, ground vibration units have been listed in mm/s.

In addition to the ground vibrations; overpressure, or air vibrations are generated through the direct action of the explosive venting through cracks in the rock or through the indirect action of the rock movement. In either case, the result is a pressure wave which travels through the air, measured in decibels (or dB) for the purposes of this report.

VIBRATION AND OVERPRESSURE THEORY

Transmission and decay of vibrations and overpressure can be estimated by the development of attenuation relations. These relations utilize empirical data relating measured velocities at specific separation distances from the vibration source to predict particle velocities at variable distances from the source. While the resultant prediction equations are reliable, divergence of data occurs as a result of a wide variety of variables, most notably site-specific geological conditions and blast geometry and design for ground vibrations and local prevailing climatic conditions for overpressure.

In order to circumvent this scatter and improve confidence in forecast vibration levels, probabilistic and statistical modeling is employed to increase conservatism built into prediction models, usually by the application of 95% confidence lines to attenuation data.

The attenuation relations are not designed to conclusively predict vibration levels at a specific location as a result of a specific blast design, application of this probabilistic model creates confidence that for any given scaled distance, 95% of the resultant velocities will fall below the calculated 95% regression line.

While the data still provides insight into probable vibration intensities, attenuation relations for overpressure tends to be less reliable and precise than results for ground vibrations. This is due primarily to wider variations in variables outside of the influence of the blast design which impact propagation of the vibrations. Atmospheric factors such as temperature gradients and prevailing winds (refer to Appendix B) as well as local topography can all serve to significantly alter overpressure attenuation characteristics. Fortunately, as described above, the conservatism built into the equations and the on-site vibration and overpressure monitoring performed at the quarry will aid in quantifying the vibrations and overpressures specific to the area in order to further reduce the likelihood of damage to all structures adjacent the quarry limits.

Our experience and analysis demonstrates that blast overpressure is greatest when blasting towards receptors, and blast vibrations are greatest when retreating towards the receptors.

GROUND VIBRATION LEVELS AT THE NEAREST SENSITIVE RECEPTOR

The most commonly used formula for predicting Peak Particle Velocity (PPV) is known as the Bureau of Mines (BOM) prediction formula or Propagation Law. We have used this formula to predict the PPV's at the closest house for the initial operations.

$$PPV = k\left(\frac{d}{\sqrt{w}}\right)^{e}$$

Where, PPV = the predicted peak particle velocity (mm/s)

K, e = site factors

d = distance from receptor (m)

w = maximum explosive charge per delay (kg)

The value of K and e are variable and influenced by many factors (i.e. rock type, geology, thickness of overburden, etc.). As such, these site factors are developed empirically through the measurement of vibration characteristics at the specific operations of interested.

The portion of the BOM prediction formula contained within the parentheses is referred to as the *Scaled Distance* and represents another important PPV relation. It correlates the separation distance between a blast and receptor to the energy (usually expressed as explosive weight) released at any given instant in time. The two most popular approaches are square root scaling and cube root scaling:

$$(SDSR = \frac{R}{\sqrt{W}}) \qquad (SDCR = \frac{R}{\sqrt[3]{W}})$$

Where, SDSR = Scaled distance square root method

SDCR = Scaled distance cube root method

R = Separation distance between receptor site and blast (m)

W = Maximum explosive load per delay period (kg)

Historically, square root scaling is employed in situations whereby the explosive load is distributed in a long column (i.e. blasthole) while cube root scaling is employed for point charges. In accordance with industry standard, square root scaling was adopted for ground vibration analysis for the purposes of this report.

For a distance of 710m (the standoff distance to the closest sensitive receptor for the initial Phase 1a blasting (Mid Extraction Area), namely 10148 Beaversdam Road) and a maximum explosive load per delay of 118kg (101mm diameter hole, 15m deep, 3m surface collar and 1 hole per delay), we can calculate the maximum PPV at the closest building using the following formulae:

Imperial Equations:

Oriard 50% Bound (2002)	$v = 160(\frac{D}{\sqrt{W}})^{-1.6}$
Oriard 90% Bound (2002)	$v = 242(\frac{D}{\sqrt{W}})^{-1.6}$
Quarry Production Blast (Bulletin 656 – 1971)	$v = 182(\frac{D}{\sqrt{W}})^{-1.82}$
Typical limestone Quarry (Pader report – 1995)	$v = 52.2(\frac{D}{\sqrt{W}})^{-1.38}$
Typical Coal Mine (RI8507 1980)	$v = 133(\frac{D}{\sqrt{W}})^{-1.5}$

Metric Equations:

General Blasting (Dupont)	$v = 1140(\frac{D}{\sqrt{W}})^{-1.6}$
Construction Blasting (Dowding 1998)	$v = 1326(\frac{D}{\sqrt{W}})^{-1.38}$
Agg. Quarry Blasting (Explotech 2005)	$v = 5175(\frac{D}{\sqrt{W}})^{-1.76}$
Agg. Quarry blasting (Explotech 2003)	$v = 7025(\frac{D}{\sqrt{W}})^{-1.85}$

The equations described above accommodate for a range of geological conditions. The proposed blast parameters were applied to the formulae to estimate a range of the potential vibrations to be imparted on the closest sensitive receptor behind the blast. As discussed in previous sections, the MECP guideline for blast-induced vibration is 12.5 mm/s (0.5 in/s). Appendix C demonstrates that the maximum (ie worst-case) calculated value for the vibration intensities imparted on the closest sensitive receptor based on all equations is 4.14mm/s for the initial blasting, well below the MECP guideline limit. All blasts will be monitored for overpressure and ground vibrations with blast designs adjusted in response to readings on site in order to confirm consistent compliance with established limits.

All vibration calculations and tables going forward will utilize the formula providing the worst case scenario for all geological conditions (Construction Blasting (Dowding 1998)).

An ***example*** of this calculation is as follows:

For a distance of 710m (the standoff distance to the closest sensitive receptor for the initial Phase 1a blasting, namely 10148 Beaversdam Road) and a maximum explosive load per delay of 118kg (101mm diameter hole, 15m deep, 3m surface collar and 1 hole per delay), we can calculate the maximum PPV at the closest sensitive receptor as follows:

$$ppv = 1326\left(\frac{710}{\sqrt{118}}\right)^{-1.38} = 4.14mm/s$$

As discussed in previous sections, the MECP guideline for blast-induced vibration is 12.5 mm/s (0.5 in/s). The calculated 95% predicted PPV (based on the proposed blasting data discussed above) would be 4.14mm/s, well below the MECP guideline limit. It is understood that as separation distance to the receptors decreases, adjustments to blast designs may be necessary to maintain compliance with the guideline limits.

Similarly, the above equation used to calculate PPV can be reformatted to find an approximation of the distance at which a vibration velocity of 12.5mm/s would occur if all blasting parameters are kept the same as used in the equation above:

$$12.5 = 1326\left(\frac{d}{\sqrt{118}}\right)^{-1.38} = 319.0m$$

The above result suggests that design modifications to the above preliminary design of an explosive load of 118kg/delay would be required once blasting operations encroach to within 319m of any sensitive receptor. Fortunately, vibration data will be continually collected and analyzed as the sensitive receptors are approached in order to confirm the requirement for any design modifications. An abundance of design modifications are available which would readily maintain vibration intensities below guideline limits. This is based on conservative assumptions which will be confirmed through monitoring.

Given the separation distances that will be involved at the proposed Uppers Quarry, Table 2 below provides initial guidance on maximum loads per delay based on various separation distances. The following maximum loads per delay were derived from the equation for ground vibrations listed above and are based on a maximum intensity of 12.5mm/s:

Separation distance between sensitive receptor and closest borehole (meters)	Maximum recommended explosive load per delay (Kilograms)
500	290
450	235
400	185
350	140
300	105
250	70
200	45
150	25
100	11

Table 2: Maximum Loads per Delay to Maintain 12.5mm/s at Various Separation Distances

It is noteworthy that the above values are typically conservative and are intended as a guideline only as the ground vibration attenuation equation is based on a calculated 95% regression line. Actual loads can be adjusted on the basis of the results of the monitoring program in place.

The closest separation distance between a sensitive receptor and the licence boundary is 63m. While blasting at this separation distance is feasible from a technological perspective, given current blasting technology and techniques, market economics will dictate the feasibility of extracting rock at lesser separation distances. Monitoring and changes in blasting designs will be required in order to confirm all blasts are within MECP guidelines when blasting comes closer to adjacent sensitive receptors.

Similarly to the paragraph above, the closest separation distance between a non-sensitive receptor (namely the utility buildings located at 4832 Thorold Townline Road) and the extraction limits of the licence is 40m. Using the above equation and keeping the same blasting parameters with a suggested limit of 50mm/s, the calculation would suggest that once blasting encroaches to 116m removed from the utility buildings, modifications may be required.

OVERPRESSURE LEVELS AT THE NEAREST SENSITIVE RECEPTOR

It is unusual for overpressure to reach damaging levels and when it does, the evidence is typically immediate and obvious in the form of broken windows in the area. However, overpressure remains of interest due to its ability to travel further distances as well as cause audible sounds and excitation in windows and walls.

Air overpressure decays in a known manner in a uniform atmosphere, however, a uniform atmosphere is not a normal condition. As such, air overpressure attenuation is far more variable due to its intimate relationship with environmental influences. Air vibrations decay slower than ground vibrations with an average decay rate of 6dBL for every doubling of distance.

Air overpressure levels are analyzed using cube root scaling based on the following equation:

$$P = k\left(\frac{d}{\sqrt[3]{w}}\right)^{e}$$

Where,
- P = the peak overpressure level (psi – imperial, Pa, dB - metric)
- K, e = site factors
- d = distance from receptor (ft – imperial, m - metric)
- w = maximum explosive charge per delay (lbs – imperial, kg - metric)

The value of K and e are variable and are influenced by many factors (i.e. rock type, geology, thickness of overburden, environmental conditions, etc.). As such, these site factors are developed empirically through the measurement of overpressure characteristics at the specific operations of interested.

As discussed in previous sections, the MECP guideline for blast-induced overpressure is 128dBL. For a distance of 710m (i.e. the standoff distance to the closest sensitive receptor in front of the blast for the initial Phase 1a blasting (Mid Extraction Area), namely 10148 Beaversdam Road) and a maximum explosive load of 118kg (101mm diameter hole, 15m deep, 3m surface collar and 1 hole per delay), we can calculate the overpressure at the nearest receptor in front of the blast using the following equations:

Imperial Equations:

USBM RI8485 (Behind Blast) $$P = 0.056(\frac{D}{\sqrt[3]{W}})^{-0.515}$$

USBM RI8485 (Front of Blast) $$P = 1.317(\frac{D}{\sqrt[3]{W}})^{-0.966}$$

USBM RI8485 (Full Confined) $$P = 0.061(\frac{D}{\sqrt[3]{W}})^{-0.96}$$

Construction Average (Oriard 2005) $$P = 1(\frac{D}{\sqrt[3]{W}})^{-1.1}$$

Metric Equations:

Ontario Quarry - dB (Explotech) $$P = 159(\frac{D}{\sqrt[3]{W}})^{-0.0456}$$

Limestone - dB (Explotech) $$P = 206(\frac{D}{\sqrt[3]{W}})^{-0.1}$$

Ontario Quarry - Pa (Explotech) $$P = 1222(\frac{D}{\sqrt[3]{W}})^{-0.669}$$

Appendix C demonstrates that the maximum calculated value for the overpressure intensities imparted on the closest sensitive receptor based on all equations is 126.8 dB(L) for the initial blasting, below the MECP guideline limit. For initial blasting, all blasts should be scheduled on days with favourable weather conditions to assist in mitigating overpressure levels.

The initial blasting area is also located in the most optimal location such that the direction of extraction will only increase the distance removed from where the front face is directed towards the sensitive receptor. Every subsequent blast in Phase 1a when retreating to the East and South from the initial Phase 1a sinking cut area will result in progressively lower overpressure results. All blasts will be monitored for overpressure and ground vibrations with blast designs adjusted in response to readings on site in order to maintain consistent compliance with established limits.

Based on the above calculation and the assumed blast parameters, and the conservatism built into the equations, overpressures from blasting operations can remain compliant with the MECP NPC 119 guideline limit of 128dBL. The design method of retreat has been planned so as to direct overpressures generated as much as practicable in the direction of vacant lands. All overpressure calculations and tables going forward will utilize the formula providing the worst case scenario for all geological conditions (Ontario Quarry – Pa (Explotech)).

We reiterate that air overpressure attenuation is far more variable due to its intimate relationship with environmental influences and as such, the equation employed is less reliable than that developed for ground vibration. Overpressure monitoring performed on site shall be used to guide blast design as it pertains to the control of blast overpressures. As demonstrated in Appendix B, prevailing winds during quarry operational periods are predominantly out of the Southwest.

The overpressure equation used to calculate PSPL can be reformatted to find an approximation of the distance at which an overpressure of 128 dB(L) would occur. If all blasting parameters are kept the same as above, a distance of 580m from the closest sensitive receptor in front of the blast would have a calculated overpressure of 128db(L). Once again, the on-site monitoring program will accurately delineate the overpressure intensities and provide guidance for the timing for any design changes.

Given the correlation between overpressure and environmental conditions as stated previously, care must be taken to avoid blasting on days when weather patterns are less favourable. Extraction directions have been selected so as to minimize overpressure impacts on adjacent receptors.

Table 3 below can be used as an initial guide showing maximum loads per delay based on various separation distances for receptors in front of the blast face. The following maximum loads per delay are derived from the air overpressure equation above and are based on a peak overpressure level of 128dB(L):

Separation distance between sensitive receptor and closest blasthole (meters)	Maximum recommended explosive load per delay (Kilograms)
1000	610
900	440
800	310
700	208
600	130
500	75

Table 3: Maximum Calculated Loads per Delay to Maintain 128dB(L) at Various Separation Distances for Receptors in Front of the Face

We note that the above values are conservative and are intended as a guideline only as the air overpressure attenuation is based on a calculated 95% regression line. Actual loads employed shall be based on the results of the monitoring program in place.

ADDITIONAL CONSIDERATIONS OUTSIDE OF THE BLAST IMPACT ANALYSIS SCOPE

The following headings are addressed for general information purposes and are not strictly required as part of the scope of the Blast Impact Analysis as required under the ARA to assess compliance with MECP NPC-119 guidelines. Considerations for the TC Energy Pipeline and Hydro One transmission towers can be expanded upon under separate cover with direct input from the owners as required. The hydrogeological study prepared by WSP as part of the licence application will address residential water wells in detail. Flyrock control is addressed at the operational level given significant influences related to blast design, geology and field accuracy which render concrete recommendations related to control inappropriate at the licencing phase. Considerations for aquatic species in the existing watercourse are further addressed in the Stantec report.

TC ENERGY HIGH PRESSURE NATURAL GAS PIPELINE

A TC Energy High Pressure Natural Gas Pipeline runs adjacent to the Northwest corner of the proposed quarry limits (refer to Appendix A). The MECP guideline for blast-induced vibration (12.5mm/s) does not apply to pipelines as they are not classified as sensitive receptors. Two (2) welded steel pipelines exist within the TC Energy right of way (ROW), one 20” and one 36” diameter pipeline. TC Energy Policy employs a 50mm/s vibration limit for welded steel pipelines. Based on the proposed Operations Plan for the Uppers Quarry, initial blasting operations are anticipated to be required approximately 345m from the subject pipeline, however, will reach as close as 7m throughout the course of extraction.

Applying the equation from Predicated Vibration Limits at the Nearest Sensitive Receptor, for a distance of 345m (the conservative standoff distance to the pipeline for the initial blasting (Mid Extraction Area)) and a maximum explosives load per delay of 118kg (101mm diameter hole, 15m deep, 3m surface collar and 1 hole per delay), we can calculate the maximum PPV at the pipeline as follows for the initial blast:

$$ppv = 1326\left(\frac{345}{\sqrt{118}}\right)^{-1.38} = 11.22mm/s$$

The calculated 95% predicted PPV (based on the proposed blasting data discussed above) would be 11.22mm/s, well below the TC Energy limit of 50mm/s for a steel welded pipeline located adjacent to the proposed quarry.

While this initial value resides below the required threshold, it is anticipated that design modifications will be necessary to maintain compliance as the separation distance to the pipeline decreases and column loads increase. Fortunately, a variety of blast design alternatives are available to accomplish this including but not limited to reductions in blast hole diameter, change in explosives types, adjustment in bench heights and decking of holes.

We do note that the TC Energy Blasting Specification requires the presence of a vibration monitoring program when blasting operations are to be conducted within 100m of a pipeline. The proposed Operational Plan dictates that blasting is to encroach within 7m of the ROW and as such, it is a recommendation of this report that an independent third party firm be retained to conduct vibration monitoring on this pipeline when separation encroaches within 100m of the pipeline or when calculations suggest ground vibrations in excess of 35mm/s as measured at the pipeline are anticipated. The results of this monitoring program will determine what blast design alterations shall be necessary as the separation distance to the subject pipeline decreases. Walker will adhere to the guidelines set by TC Energy with respect to blasting in proximity of the gas lines.

FLYROCK

Flyrock is the term used to define rocks which are propelled from the blast area by the force of the explosion. This action is a predictable and necessary component of a blast and requires that every blast have an exclusion zone established within which no persons or property which may be harmed are permitted.

Government regulations strictly prohibit the ejection of flyrock off of a quarry property. The regulations regarding flyrock are enforced by the Ministries of Natural Resources, Environment and Labour. In the event of an incident where flyrock does leave a site, the punitive measures include suspension / revocation of licences and fines to both the blaster and quarry owner / operator. Fortunately, flyrock incidents are extremely rare due to the possible serious consequences of such an event. It is in the best interest of all, stakeholders and non-stakeholders, to ensure that dangerous flyrock does not occur. Through proper blast planning and design, it is possible to control and mitigate the possibility for flyrock.

THEORETICAL HORIZONTAL FLYROCK CALCULATIONS

Flyrock occurs when explosives in a hole are poorly confined by the stemming or rock mass and the high pressure gas breaks out of confinement and launches rock fragments into the air. The three primary sources of fly rock are as follows:

- **Face burst:** Lack of confinement by the rock mass in front of the blast hole results in fly rock in front of the face.
- **Cratering:** Insufficient stemming height or weakened collar rock results in a crater being formed around the hole collar with rock projected in any direction.
- **Stemming Ejection:** Poor stemming practice can result in a high angle throw of the stemming material and loose rocks in the blasthole wall and collar.

The horizontal distance flyrock can be thrown (L_H) from a blast hole is determined using the expression:

$$L_H = \frac{V_o^{\,2} Sin 2\theta_0}{g} \qquad [1]$$

where: V_o = launch velocity (m/s)

θ_0 = launch angle (degrees)

g = gravitational constant (9.8 m/s^2)

The theoretical maximum horizontal distance fly rock will travel occurs when θ_0 = 45 degrees, thereby yielding the equation:

$$L_{H\max} = \frac{V_o^{\ 2}}{g} \qquad [2]$$

The normal range of launch velocity for blasting is between 10m/s - 30m/s. To calculate the launch velocity of a blast the following formula is used:

$$V_o = k\left(\frac{\sqrt{m}}{B}\right)^{1.3} \qquad [3]$$

where: k = a constant

m = charge mass per meter (kg/m)

B = burden (m)

By combining equations 2 and 3 and taking into account the different sources of fly rock, the following equations can be used to calculate the maximum fly rock thrown from a blast:

Face burst:

$$L_{H\max} = \frac{k^2}{g} * \left(\frac{\sqrt{m}}{B}\right)^{2.6}$$

Cratering:

$$L_{H\max} = \frac{k^2}{g} * \left(\frac{\sqrt{m}}{SH}\right)^{2.6}$$

Stemming Ejection: $$L_{H\max} = \frac{k^2}{g} * \left(\frac{\sqrt{m}}{SH}\right)^{2.6} Sin2\theta$$

where:
θ = drill hole angle
L_{hmax} = maximum flyrock throw (m)
m = charge mass per meter (kg/m)
B = burden (m)
SH = stemming height (m)
g = gravitational constant
k = a constant

The range for the constant k is 13.5 for soft rocks and 27 for hard rocks. Given the proposed licence area is predominantly limestone, we have applied a k value of 21. The explosive density is assigned to be 1.2 g/cc for emulsion products and the drill hole angles are assumed to be 90 degrees (i.e. vertical).

For calculation purposes, we have applied the initial blasting parameters which utilize 101mm (4") diameter holes on a 3.05m x 3.05m (10' x 10') pattern, with a lift height of 15m (49') and a collar length of 3.0m (10'). The following does not apply to the sinking cut which will require highly specialized designs and additional considerations for flyrock. Based on a free face blast, maximum anticipated horizontal flyrock projection distances are calculated as follows in Table 4:

Table 4 – Maximum Flyrock Horizontal

***Collar* Lengths** (m)	**Maximum Throw Face Burst** (m)	**Maximum Throw Cratering and Stemming Ejection** (m)
1.5	48	302
2.0	48	143
2.5	48	80
3.0	48	50
3.5	48	33

Different collar lengths are displayed in the table above to account for over or under loaded holes. As demonstrated with these various collar lengths, any deviation, no matter how slight, can greatly affect these maximum values. The current proposed initial blasting parameters have the potential to send flyrock 50m assuming all holes achieve the designed collar lengths of 3.0m. Blast mats or sand can be placed on top of the initial blast to further reduce the distance for potential flyrock.

Through proper blast design and diligence in inspecting the geology before every blast, flyrock can readily be maintained within the quarry limits. It may be necessary to increase collars and adjust designs accordingly when blasting along the perimeter to accommodate the reduced deportation distance to receptors and to maintain flyrock within the property limits. The operational plan for the quarry has been designed to retreat towards the closest receptors thereby projecting flyrock and overpressures away from the receptors.

TRANSMISSION AND HYDRO TOWERS

Transmission towers (Namely the Hydro One Corridor) runs parallel to the Southern limits of the proposed quarry licence noted on the proposed Operational Plan (refer to Appendix A). The MECP guideline for blast-induced vibration (12.5mm/s) does not apply to transmission/hydro towers as they are not classified as sensitive receptors. In order to safeguard the integrity of these structures, Hydro One has set a vibration limit of 50mm/s at the foundations of the transmission towers.

As per direction from Hydro One, calculations will be based on the 50mm/s limit. The tower shall be monitored for ground vibration and overpressure when vibration calculations suggest vibrations in excess of 35mm/s at the tower base. Based on the proposed Operations Plan for the Uppers Quarry, initial blasting operations are anticipated to be approximately 530m from the closest tower, however, will reach as close as 30m throughout the course of extraction at the Southern limits of Phase 1b and Phase 5.

Applying the equation from Predicated Vibration Limits at the Nearest Sensitive Receptor, for a distance of 530m (the conservative standoff distance to the transmission tower for the initial blasting) and a maximum explosives load per delay of 118kg (101mm diameter hole, 15m deep, 3m surface collar and 1 hole per delay), we can calculate the maximum PPV at the transmission tower for the initial blast as follows:

$$ppv = 1326\left(\frac{530}{\sqrt{118}}\right)^{-1.38} = 6.2mm/s$$

The calculated 95% predicted PPV (based on the proposed blasting data discussed above) would be 6.2mm/s, well below the limit of 50mm/s. While this value resides below the 50mm/s threshold, it is anticipated that design modifications will be necessary to maintain compliance as the separation distance to some of the towers decreases and column loads increase. Fortunately, a variety of blast design alternatives are available to accomplish this including but not limited to reductions in blast hole diameter, change in explosives types, adjustment in bench heights and decking of holes.

RESIDENTIAL WATER WELLS

Possible impacts to the water quality and production capacity of groundwater supply wells is a common concern for residents near blasting operations. Complaints related to changes in water quality often include the appearance of turbidity, water discolouration and changes in water characteristics (including nitrate, e-coli, and coliform contamination). Complaints regarding water production most often involve loss of quantity production, air in water and damage to well screens and casings. A review of research and common causes of these problems indicates that most of these concerns are not related to blasting and can be shown to be the direct impact of environmental factors and poor well construction and maintenance.

There is significant research and scientific substantiation demonstrating that outside of the immediate radius of approximately 20-25 blasthole diameters from a loaded hole, there is no permanent ground displacement resulting from a blast. As such, barring blasting activity within several meters of an existing well, the probability of damage to residential wells is essentially non-existent.

Despite the scientific support for the above conclusion, numerous studies have been performed to verify the validity of this statement. These studies have investigated the effects of blasting on varied well configurations and in varied geological mediums to permit conclusions to be readily extrapolated to diverse blasting operations. The conclusion of these studies has confirmed that with the exception of possible temporary increases in turbidity, blasting operations did not result in any permanent impact on wells outside of the immediate blast zone of the blast until vibrations levels reached exceedingly high intensities. Applying universally accepted threshold levels for ground vibrations eliminates the possibility for any long term adverse effects on wells in the vicinity of blasting operations.

In a study by Froedge (1983), blast vibration levels of up to 32.3mm/s were recorded at the bottom of a shallow well located at a distance of 60 meters (200 feet) from an open pit blast. There was no report of visible damage to the well nor was there any change in the water pumping flow rate. This study concluded that the commonly accepted limit of 50mm/s PPV level is adequate to protect wells from any damage. We reiterate, the current guideline limit for vibrations from quarry and mining operations is 12.5mm/s.

Rose et al. (1991) studied the effect of blasting in close proximity to water wells near an open pit mine in Nevada, USA. Blasts of up to 70 kilograms of explosives per delay period were detonated at a distance of 75 meters (245 feet) from a deep water well. There was no reported visible damage to the well. Fluctuations in water level and flow rate were evident immediately after the blast. However, the well water level and flow rate quickly stabilized.

The U.S. Bureau of Mines conducted a study (Robertson et al., 1990) to determine the changes in well capacity and water quality. This involved pumping from wells before and after nearby blasting. One experiment with a well in sandstone showed no change in well capacity after blasts induced PPV's at the surface of 84mm/s and there was no change in water level after PPV's of 141mm/s, well above the current guideline limit of 12.5mm/s.

Matheson et al. (1997) brought together available information on the most common complaints, the possible causes of the complaints and the relation between blasting and the complaint causes. This study yet again reaffirmed the fact that the attribution of well problems to blast sources are unfounded.

The MECP vibration limit of 12.5mm/s effectively excludes any possibility of damage to residential water wells. Based on available research and our extensive experience in Ontario quarry blasting, blasting at the Upper's Quarry will induce no permanent adverse impacts on the residential water wells on properties surrounding the site.

BLAST IMPACT ON ADJACENT WATERCOURSES

The detonation of explosives in or near water can produce compressive shock waves which initiate damage to the internal organs of fish in close proximity, ultimately resulting in the death of the organism. Additionally, ground vibrations imparted on active spawning beds have the ability to adversely impact the incubating eggs and spawning activity. In an effort to alleviate adverse impacts on fish populations as a result of blasting, the Department of Fisheries and Oceans (DFO) developed the Guidelines for the Use of Explosives In or Near Canadian Fisheries Waters (1998). This publication establishes limits for water overpressure and ground vibrations which are intended to mitigate impacts on aquatic organisms while providing sufficient flexibility for blasting to proceed. Specifically, water overpressures are to be limited to 100kPa and, in the presence of active spawning beds, ground vibrations at the bed are to be limited to 13mm/s.

An existing watercourse currently runs in a North/South direction through the middle of Phase 3a and the Western sections of Phases 4 and 5. The operational plan has proposed for the realignment of the existing watercourse as part of the licence but will remain in its current location during blasting in Phases 1a and 1b. The operational plan shows the existing watercourse alignment, prior to its realignment, at an approximate 26m setback distance from all surrounding phases. Based on these separation distances and our experience on similar operations, water overpressures generated by the blasting will reside below the DFO 100kPa guideline limit and will have no impact on the adult fish populations present.

As per a preliminary document completed by Stantec, one of the fish species identified (Pike) in the adjacent watercourse have two (2) distinct spawning areas but technically anywhere that vegetation is flooded could be potential spawning habitat. The closest area of potential spawning lies approximately 208m from the initial Phase 1a (Mid Extraction Area) Sinking Cut Area blasting operations. The spawning time for the identified fish species in the adjacent watercourses has been established from March – July. As such, active spawning beds present would be subject to the DFO guideline vibration limit of 13mm/s. During spawning season, vibration monitoring will be required at the shoreline adjacent the closest spawning area on the blast side of the water body in order to confirm compliance with DFO limits for ground vibration.

Table 5 below is presented as initial guidance showing maximum permissible loads per delay based on various separation distances from spawning beds. The following maximum loads per delay are derived from the equation for ground vibrations listed earlier in this report and are based on a maximum vibration intensity of 13.0mm/s as experienced at the active spawning habitat:

Separation distance between possible spawning bed and closest borehole (meters)	Maximum recommended explosive load per delay (Kilograms)
500	305
450	245
400	195
350	150
300	110
250	75
200	49
150	27
100	12
75	6.5
50	3
30	1

Table 5: Maximum Loads per Delay to Maintain 13.0mm/s at Various Separation Distances

The generation of suspended solids within the watercourse as a result of the blasting activities will be negligible and grossly subordinate to suspended solids generated as a result of spring runoff and rain activity.

RECOMMENDATIONS

It is recommended that the following conditions be applied for all blasting operations at the proposed Upper's Quarry:

1. An attenuation study shall be undertaken by an independent blasting consultant during the first 12 months of operation in order to obtain sufficient quarry data to confirm the initial guideline parameters and assist in refining future blast designs.

2. All blasts shall be monitored for both ground vibration and overpressure at the closest privately owned sensitive receptors adjacent the site, or closer, with a minimum of two (2) instruments – one installed in front of the blast and one installed behind the blast.

3. Blasts shall be designed to maintain vibrations below 13mm/s at the location of the closest identified active spawning bed as per DFO guidelines. When blasting during active spawning season, a minimum of one supplemental vibration monitor shall be installed on the shoreline closest to the spawning bed to confirm the vibration levels.

4. The guideline limits for vibration and water overpressure shall adhere to standards as outlined in the Guidelines For the Use of Explosives In or Near Canadian Fisheries Waters (1998) or any such document, regulation or guideline which supersedes this standard.

5. All blasts shall be monitored for ground vibration at the adjacent TC Energy High Pressure Natural Gas Pipeline when blasting within 100m of the pipeline or when calculations suggest vibrations in excess of 35mm/s.

6. Blasts shall be designed to maintain vibrations at the transmission towers in the Hydro One Corridor below 50mm/s or any such document, regulation or corporate policy in effect at the time. When vibration calculations suggest vibrations at the towers may exceed 35mm/s, the towers shall be monitored for ground vibration.

7. Blasts shall be designed to maintain vibrations at the 4832 Thorold Townline Road utility buildings below 50mm/s. When vibration calculations suggest vibrations at the utility buildings may exceed 35mm/s, the buildings shall be monitored for ground vibration.

8. The guideline limits for ground vibration and air overpressure shall adhere to standards as outlined in the Model Municipal Noise Control By-law

publication NPC 119 (1978) or any such document, regulation or guideline which supersedes this standard.

9. Orientation of the aggregate extraction operation will be designed and maintained so that the direction of the overpressure propagation will be away from structures as much as possible.

10. Blast designs shall be continually reviewed with respect to fragmentation, ground vibration and overpressure. Blast designs shall be modified as required to maintain compliance with current applicable guidelines and regulations.

11. Detailed blast records shall be maintained in accordance with current industry best practices.

The blast parameters described within this report are supported by the modelling in the attached appendices. As the quarry progresses and as site-specific data is collected from the on-going operation, the blast parameters can be refined, as necessary, to maintain continual compliance with MECP Guidelines.

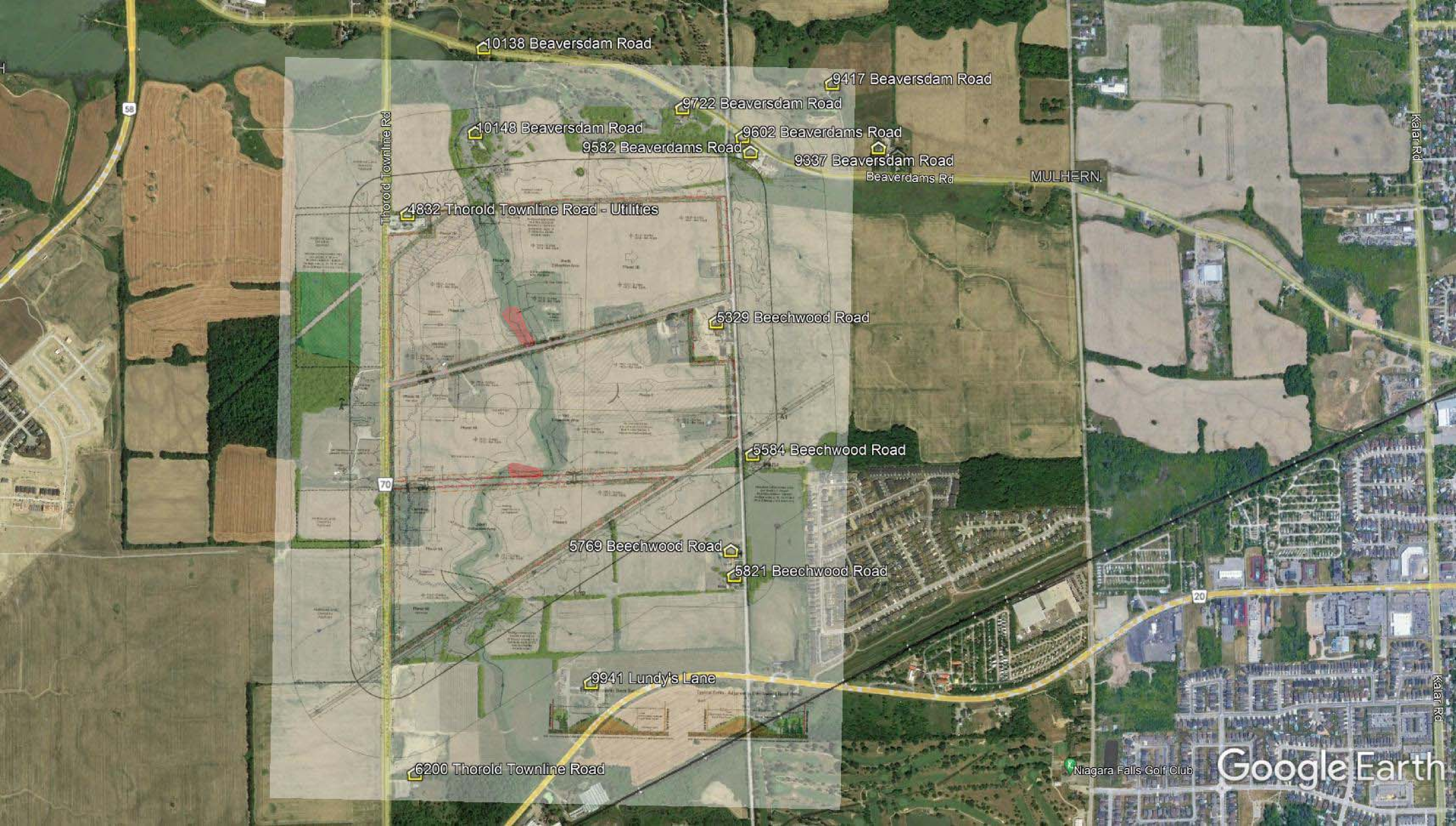
CONCLUSION

The blast parameters described within this report will provide a good basis for the initial blasting operations at this location. As site specific blast vibration and overpressure data becomes available, it will be possible to refine these parameters on an ongoing basis.

Blasting operations required for operations at the proposed Uppers Quarry site can be carried out safely and within governing guidelines set by the Ministry of the Environment, Conservations and Parks.

Modern blasting techniques will permit blasting to take place with explosives charges below allowable charge weights ensuring that blast vibrations and overpressure will remain minimal at the nearest receptors.

Appendix A


10138 Beaversdam Road
9417 Beaversdam Road
9722 Beaversdam Road
10148 Beaversdam Road
9602 Beaverdams Road
9582 Beaverdams Road
9337 Beaversdam Road
Beaverdams Rd
MULHERN
Kalar Rd
Thorold Townline Rd
4832 Thorold Townline Road - Utilities
5329 Beechwood Road
5584 Beechwood Road
5769 Beechwood Road
5821 Beechwood Road
9941 Lundy's Lane
6200 Thorold Townline Road
58
70
20
Niagara Falls Golf Club
Google Earth

Appendix B

EXPLOTECH

Uppers Quarry

PREVAILING METEOROLOGICAL CONDITIONS

Medians provided by Environment Canada
Canadian Climate Normals 1981-2010
St Catherines – Municipal Airport

Date	Wind Direction	Max Hourly Wind Velocity Km/h	Temperature (Deg Celsius)
January	SW	89	-3.8
February	E	63	-2.9
March	SW	74	1.1
April	SW	74	7.4
May	SW	65	13.7
June	SW	65	19.0
July	SW	63	21.9
August	W	59	20.8
September	W	53	16.6
October	SW	63	10.4
November	SW	70	4.6
December	SW	70	-0.9

Appendix C

Ground Vibrations

Imperial Equations

Equation 1	Equation 2	Equation 3	Equation 4	Equation 5
Oriard 50% Bound (2002)	Oriard 90% Bound (2002)	Typical Production Blast (Bulletin 656 – 1971)	Typical limestone Quarry (Pader report – 1995)	Typical Coal Mine (RI8507 1980)
$v = 160\left(\frac{D}{\sqrt{W}}\right)^{-1.6}$	$v = 242\left(\frac{D}{\sqrt{W}}\right)^{-1.6}$	$v = 182\left(\frac{D}{\sqrt{W}}\right)^{-1.82}$	$v = 52.2\left(\frac{D}{\sqrt{W}}\right)^{-1.38}$	$v = 133\left(\frac{D}{\sqrt{W}}\right)^{-1.5}$

Metric Equations

Equation 1	Equation 2	Equation 3	Equation 4
DuPont General (1968)	Construction Blasting (Dowding 1998)	Agg. Quarry Blasting (Explotech 2005)	Agg. Quarry blasting (Explotech 2003)
$v = 1140\left(\frac{D}{\sqrt{W}}\right)^{-1.6}$	$v = 1326\left(\frac{D}{\sqrt{W}}\right)^{-1.38}$	$v = 5175\left(\frac{D}{\sqrt{W}}\right)^{-1.76}$	$v = 7025\left(\frac{D}{\sqrt{W}}\right)^{-1.85}$

D (m)	W (Kg)	PPV1 (mm/s)	PPV2 (mm/s)	PPV3 (mm/s)	PPV4 (mm/s)	PPV5 (mm/s)	PPV1 (mm/s)	PPV2 (mm/s)	PPV3 (mm/s)	PPV4 (mm/s)
710	118	**1.4**	**2.2**	**0.5**	**1.4**	**1.9**	**1.4**	**4.1**	**3.3**	**3.1**

Air Overpressure

Imperial Equations

Equation 1	Equation 2	Equation 3	Equation 4
USBM RI8485 (Behind Blast)	USBM RI8485 (Front of Blast)	USBM RI8485 (Full Confined)	Construction Average
$P = 0.056\left(\frac{D}{\sqrt[3]{W}}\right)^{-0.515}$	$P = 1.317\left(\frac{D}{\sqrt[3]{W}}\right)^{-0.966}$	$P = 0.061\left(\frac{D}{\sqrt[3]{W}}\right)^{-0.96}$	$P = 1\left(\frac{D}{\sqrt[3]{W}}\right)^{-1.1}$

Metric Equations

Equation 1	Equation 2	Equation 3
Ontario Quarry (Explotech 2013)	Limestone (Explotech 2011)	Ontario Quarry (Explotech 2012)
$P = 159\left(\frac{D}{\sqrt[3]{W}}\right)^{-0.0456}$	$P = 206\left(\frac{D}{\sqrt[3]{W}}\right)^{-0.1}$	$P = 1222\left(\frac{D}{\sqrt[3]{W}}\right)^{-0.669}$

D (m)	W (Kg)	OP1 (dB)	OP2 (dB)	OP3 (dB)	OP4 (dB)	OP1 (dB)	OP2 (dB)	OP3 (dB)
710	118	**119.3**	**123.6**	**97.3**	**114.4**	**126.7**	**125.3**	**126.8**

Appendix D

Robert J. Cyr, P. Eng.

Principal, Explotech Engineering Ltd.

EDUCATION

Bachelor of Applied Science,
Civil Engineering, Queen's University

PROFESSIONAL AFFILIATIONS

Association of Professional Engineers of Ontario (APEO)
Association of Professional Engineers and Geoscientists of BC (APEG)
Association of Professional Engineers, Geologists and Geophysicists of Alberta
Association of Professional Engineers and Geoscientists of New Brunswick
Association of Professional Engineers of Nova Scotia
Association of Professional Engineers and Geoscientists Manitoba
Professional Engineers and Geoscientists Newfoundland and Labrador
Northwest Territories and Nunavut Association of Professional Engineers (NAPEG)
International Society of Explosives Engineers (ISEE)
Ontario Stone Sand & Gravel Association (OSSGA)
Surface Blaster Ontario Licence 450109

SUMMARY OF EXPERIENCE

Over thirty five years experience in many facets of the construction and mining industry has provided the expertise and experience required to efficiently and accurately address a comprehensive range of engineering and construction conditions. Sound technical training is reinforced by formidable practical experience providing the tools necessary for accurate, comprehensive analysis and application of feasible solutions. Recent focus on vibration analysis, blast monitoring, blast design, damage complaint investigation for explosives consumers and specialized consulting to various consulting engineering firms.

PROFESSIONAL RECORD

2001 – Present	-Principal, Explotech Engineering Ltd.
1996 – 2001	-Leo Alarie & Sons Limited - Project Engineer/Manager
1993 – 1996	-Rideau Oxford Developments Inc. – Project Manager
1982 – 1993:	-Alphe Cyr Ltd. – Project Coordinator/Manager

EXPLOTECH

Specialists in Explosives, Blasting and Vibration
Consulting Engineers

Mitch Malcomson, P.Eng.

Consulting Engineer, Explotech Engineering Ltd.

EDUCATION

Bachelor of Engineering,
Civil Engineering with Concentration in Business Management,
Carleton University

PROFESSIONAL AFFILIATIONS

Association of Professional Engineers of Ontario (APEO)
Association of Professional Engineers and Geoscientists of BC (APEG)
International Society of Explosives Engineers (ISEE)
Ontario Stone Sand and Gravel Association (OSSGA)

SUMMARY OF EXPERIENCE

A Consulting Engineer and Project Manager for Explotech Engineering Ltd., Mitch holds a Bachelor of Engineering degree from Carleton University in Civil Engineering with a Concentration in Business Management. Mitch has strong analytical, technical, business and leadership skills. As a Project Manager, Mitch is responsible for operational strategies, scheduling and contract procurement. As a Consulting Engineer, the technical responsibilities include detailed blast designs, blast investigations and reviews, implementation of vibration monitoring programs, development of monitoring equipment/ technologies and building assessments for construction and the drilling and blasting portions of mining, quarrying and construction projects across Canada.

PROFESSIONAL RECORD

2008 – Present - Consulting Engineer / Project Manager, Explotech Engineering Ltd.

EXPLOTECH

Specialists in Explosives, Blasting and Vibration
Consulting Engineers

Andrew Campbell, P.Eng.

Explotech Engineering Ltd.

EDUCATION & QUALIFICATIONS

Bachelor of Engineering,
Mechanical Engineering, Carleton University

Advanced and Expert (Industry) CadnaA Modelling
DataKustik, Mississauga, Ontario

PROFESSIONAL AFFILIATIONS

Association of Professional Engineers of Ontario (APEO)
International Society of Explosive Engineers (ISEE)

SUMMARY OF EXPERIENCE

An engineer working for Explotech Engineering Ltd., Andrew holds a Bachelor of Engineering degree in Mechanical Engineering and has strong analytical, technical, and interpersonal skills. A proven leader in collaborative environments, Andrew is comfortable managing projects, specifying details, and communicating internally and externally. With a focus on blast designs, blast impact analyses, noise monitoring and modelling, damage complaint investigations, vibration analysis, and blast monitoring, Andrew has applied these skills across Canada.

PROFESSIONAL RECORD

2018 – Present	- Engineer, Explotech Engineering Ltd.
2013 – 2018	- Technician / EIT, Explotech Engineering Ltd.
2012 – 2012	- Ride Technician, Canada's Wonderland

EXPLOTECH

Specialists in Explosives, Blasting and Vibration
Consulting Engineers

Michael Tobin, P.Eng.

Explotech Engineering Ltd.

EDUCATION

Bachelor of Applied Science,
Geological Engineering, Queen’s University

PROFESSIONAL AFFILIATIONS

Association of Professional Engineers of Ontario (APEO)
International Society of Explosives Engineers (ISEE)

SUMMARY OF EXPERIENCE

An engineer working for Explotech Engineering Ltd., Michael holds a Bachelor of Applied Science degree from Queen’s University in Geological Engineering. Michael has strong analytical, technical, and interpersonal skills. Recent projects have focused on blast monitoring, vibration analysis, job estimation, damage complaint investigation and blast design.

PROFESSIONAL RECORD

2021 – Present - Engineer, Explotech Engineering Ltd.

2017 – 2021 - Technician, Explotech Engineering Ltd.

Appendix E

EXPLOTECH

Blasting Terminology

ANFO:	Ammonium Nitrate and Fuel Oil – explosive product
ANFO WR:	Water resistant ANFO
Blast Pattern:	Array of blast holes
Body hole:	Those blast holes behind the first row of holes (Face Holes)
Burden:	Distance between the blast hole and a free face
Column:	That portion of the blast hole above the required grade
Column Load:	The portion of the explosive loaded above grade
Collar:	That portion of the blast hole above the explosive column, filled with inert material, preferably clean crushed stone
Face Hole:	The blast holes nearest the free face
Overpressure:	A compressional wave in air caused by the direct action of the unconfined explosive or the direct action of confining material subjected to explosive loading.
Peak Particle Velocity:	The rate of change of amplitude, usually measured in mm/s or in/s. This is the velocity or excitation of the particles in the ground resulting from vibratory motion.
Scaled distance:	An equation relating separation distance between a blast and receptor to the energy (usually expressed as explosive weight) released at any given instant in time.
Sensitive Receptor:	Sensitive land use may include recreational uses which are deemed by the municipality or provincial agency to be sensitive; and/or any building or associated amenity area (i.e. may be indoor or outdoor space) which is not directly associated with the industrial use, where humans or the natural environment may be adversely affected by emissions generated by the operation of a nearby industrial facility. For example, the building or amenity area may be associated with residences, senior citizen homes, schools,

day care facilities, hospitals, churches and other similar institutional uses, or campgrounds.

Spacing:	Distance between blast holes
Stemming:	Inert material, preferably clean crushed stone applied into the blast hole from the surface of the rock to the surface of the explosive in the blast hole.
Sub-grade:	That portion of the blast hole drilled band loaded below the required grade
Toe Load:	The portion of explosive loaded below grade

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